

# Modeling multilayered wire strands, a strategy based on 3D finite element beam-to-beam contacts - Part II: Application to wind-induced vibration and fatigue analysis of overhead conductors

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## Abstract

Wind-induced loads cause electrical transmission line fatigue. Evaluation procedures consider descriptors such as deflection amplitude ( $Y_b$ ) and far-field vibration ( $f_{y_{max}}$ ), which cannot relate endurance limits and wire loads. The investigation uses the finite element (FE) strategy developed in part I to study Aluminum Conductor Steel Reinforced (ACSR) submitted to wind-induced loads. The analysis underlines the  $Y_b$  and  $f_{y_{max}}$  discrepancies. A factorial design leads to a model relating them with a precision of 92%. Comparisons with experimental ACSR data indicate that fatigue predictions from the Coffin-Manson relation associated with the FE model provide realistic evaluations of service lives.

**Keywords:** Multilayered wire strands, Finite element modeling, Bending stiffness, Overhead ACSR conductor, Dynamic bending stress, Conductor fatigue

## 1. Introduction

Cyclic bending loads resulting from wind-induced vibrations near restraining fixtures may compromise the integrity of cable-supported structures [1,2]. They also particularly affect overhead electrical transmission lines [3]. In fact, Aeolian vibrations are among the main causes of conductor fatigue damage in transmission lines [4]. Hence, a careful evaluation of their impacts on local stress distributions represents an essential exercise. However, predicting the load severity is a complex endeavor as stranded assemblies involve multiple wire contact interactions [5].

The present study exploits the finite element modeling strategy developed and validated in Part I of this two-paper series [6] to analyze the response of wire strands submitted to cyclic bending loads. Although the study focuses on ACSR (Aluminum Conductor Steel Reinforced) conductors, the proposed methodology applies to most wire strand bending problems.

In overhead conductor assemblies, severe cyclic bending loads occur near suspension clamps, vibration dampers and spacer-damper arms. At these locations, bending loads generate fretting fatigue at contact interfaces [7], and consequently, have detrimental effects on conductor service life. To estimate the dynamic load severity associated with specific vibration levels, industry standards consider fatigue indicators such as the alternating bending stress ( $\sigma_a$ ) evaluated at the topmost

31 outer layer wire [4,8]. The evaluated dynamic stress must be lower than the endurance limit of the conductor measured  
32 during experimental vibration fatigue tests [4].

33 Because the geometry of the conductor is complex,  $\sigma_a$  is very hard to encapsulate in an analytical formulation. Thus,  $\sigma_a$  is  
34 still commonly estimated through a simplified model proposed by Poffenberger and Swart [9] [10]. This approach reduces  
35 the conductor/clamp configuration to a simple cantilever beam undergoing cyclic reversed deflection at its free end while  
36 being submitted to a tension load. This model neglects all internal friction effects, and considers that each wire bends  
37 independently. The Poffenberger-Swart model therefore uses a minimal theoretical flexural stiffness ( $EI_{\min}$ ) or a lower bound  
38 of  $\sigma_a$ . This approach leads to significant underestimations of real dynamic stresses. This is especially true when vibration  
39 amplitudes are small, since under small movements, the strand wires tend to act as a solid beam.

40 While the literature proposes several analytical and semi-analytical models [5,11], none of them has gained general  
41 approbation from the field industry to date. This is due in part to the fact that each of the models is based on different  
42 simplifying assumptions, which do not match all situations. For example, Giglio and Manes [12] made use of the analytical  
43 thin-rod formulation proposed by Costello [13] and ignored inter-wire friction to predict the fatigue life of wire ropes  
44 subjected to axial loads. In their study, Argatov et al. [14] addressed the bending over sheave fatigue wear of wire ropes  
45 using a model based on Archard law in which the influence of strand kinematic on wire local stresses was simply omitted.

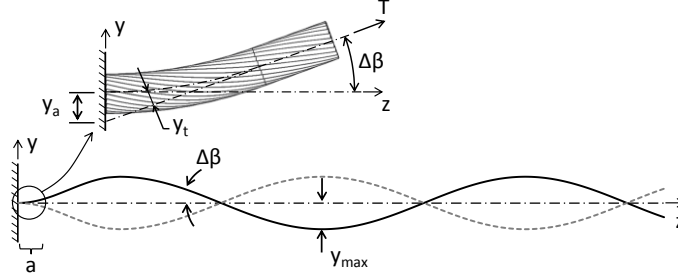
46 Modern computer capacities now allow the development of more efficient numerical tools for multilayered wire strand  
47 analysis. Through a 3D discretization of each wire with beam elements, the modeling strategy put forward in Part I [6] avoids  
48 most of the above common simplifying assumptions, and all types of inter-wire contact interactions are integrated via a line-  
49 to-line contact algorithm.

50 After a brief summary of the prevailing analytical formulations in Section 2, Section 3 of this second part of the paper  
51 series compares predicted dynamic deflection values with published experimental results, and demonstrates the precision of  
52 the modeling strategy in Part I. Next, Section 4 develops and validates a tool based on a factorial design establishing the  
53 connection between the standard stress parameters used in practice to assess the load severity. Section 5 describes the  
54 influence of the internal friction forces on alternating stress ( $\sigma_a$ ). Finally, Section 6 integrates the  $\sigma_a$  FE model predictions into  
55 a fatigue damage analysis to obtain a direct assessment of the bending load severity.

## 56 57 **2. Theoretical approach**

58 To facilitate comparison, the following briefly presents the analytical estimation approach of  $\sigma_a$ .

59 The  $\sigma_a$  formulation proposed by Poffenberger and Swart [9] considers a straight conductor with fixed ends, submitted to  
60 standing wave vibrations (Fig. 1). It also assumes that close to its fixed ends, the conductor deflection departs from the  
61 assumed sine shape; the deflection curve asymptotically progresses from a horizontal line (at the clamped end) to a sine-  
62 shape loop section (Fig. 1).



63  
64 **Fig. 1 - Schematization of conductor standing wave vibrations (reproduced from EPRI (2006))**  
65

66 Eq. (2.1) defines the strand curvature, while neglecting inertial forces. In this equation, the bending moment ( $M$ ) results from  
67 the multiplication of the axial tension ( $T$ ) and the departure of the conductor deflection from the sine-shape loop ( $y_t$ ).

68 
$$\frac{d^2 y_t}{dz^2} = \frac{M}{EI} = \frac{T}{EI} y_t \quad (2.1)$$

69 For large  $z$  values,  $y_t$  tends to zero. The solution to eq.(2.1) thus becomes  $y_t = Ae^{-\sqrt{(T/EI)z}}$ , where  $A$  is a constant expressed as

70 
$$A = \frac{y(z)}{e^{-\sqrt{(T/EI)z}} - 1 + \sqrt{T/EI}z} [15].$$
 On the other hand, at  $z = 0$  the conductor slope ( $dy/dz$ ) is  $\Delta\beta$  (Fig. 1). Therefore, assuming

71 small deflection angles, the deflection becomes  $y(z) = -y_a + \Delta\beta z + y_t$  [15]. Combining these relations into eq. (2.1) leads after  
72 some simplifications to the expression given by eq. (2.2) [15].

73 
$$\left( \frac{d^2 y_t}{dz^2} \right)_{z=0} = (T/EI) A \quad (2.2)$$

74 where  $y$  is the conductor deflection amplitude measured at a distance  $z \leq a$  (Fig. 1). The approach considers independent  
75 wires  $i$ , and therefore uses a lower bound of the strand bending stiffness ( $EI$ ) (eq. (2.3)).

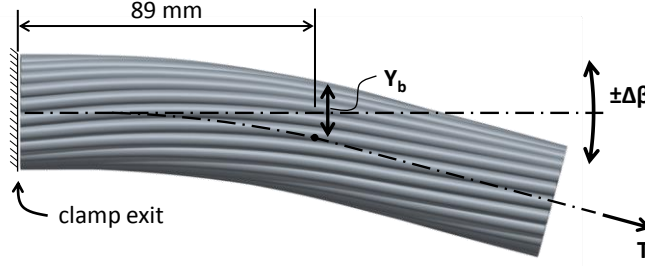
76 
$$EI = \sum_{i=1}^{i=nb \text{ wire}} E_i I_{0i} \quad (2.3)$$

77  $E_i$  is the Young modulus of the wire  $i$  material, and  $I_{0i} = \pi d_i^4 / 64$  for round wires of diameter  $d_i$ . With the conductor curvature  
78 defined by eq. (2.2), it is then possible to determine the bending stress level ( $\sigma_a$ ) at the strand fixed end ( $z = 0$ ).

79

## 80 2.1. $Y_b$ method

81 To standardize industry practice, an IEEE committee in 1966 proposed the establishment of a conductor vibration  
 82 intensity from peak-to-peak deflection ( $2y_b$ ), measured at 89 mm (3.5 in) from the clamp exit (Fig. 2). This deflection measure  
 83 is identified as the  $Y_b$  parameter.



84 **Fig. 2 - Standardized conductor dynamic bending amplitude measurement**

85 Integrating  $Y_b$  into eq.2.2 leads to eq. (2.4) also known as the *Poffenberger-Swart Formula* (PS) [9], where  $d_c$  is the conductor  
 86 diameter and  $z = 89$  mm. Eq. (2.4) establishes the alternating bending stress  $\sigma_a$ . The variable  $E_a$  represents the Young  
 87 modulus of the external layer wire material.

$$89 \quad \sigma_a = \frac{d_c E_a \left( \frac{T}{4EI} \right)}{e^{\sqrt{T/EI}z} - 1 + \sqrt{T/EI}z} Y_b \quad (2.4)$$

## 90 2.2. $fy_{max}$ method

91 The bending stress  $\sigma_a$  may also be evaluated based on the vibration frequency  $f$  and the far-field amplitude  $y_{max}$  (Fig. 1),  
 92 leading to the  $fy_{max}$  parameter.

93 At the position where the conductor deflection adopts the sine-shape loop (Fig. 1), the slope  $dy_t/dx$  corresponds to  $\Delta\beta$ , and  
 94 may be expressed as a  $fy_{max}$  function (eq. (2.5)) [4],

$$95 \quad \Delta\beta = \frac{2\pi f y_{max}}{\sqrt{T/m}} \quad (2.5)$$

96 where  $m$  is the conductor mass per unit length. eq. (2.1) may be redefined in terms of  $fy_{max}$ , eq. (2.6) [4].

$$97 \quad \left( \frac{d^2 y_t}{dz^2} \right)_{z=0} = 2\pi \sqrt{\frac{m}{EI}} f y_{max} \quad (2.6)$$

98  $\sigma_a$  is then given by eq. (2.7).

$$99 \quad \sigma_a = \pi d_c E_a \sqrt{\frac{m}{EI}} f y_{max} \quad (2.7)$$

100 *2.3. Theoretical endurance limits*

101 Based on surveys of numerous experimental conductor fatigue tests, EPRI [4] established endurance limits (at 500 M  
102 cycles) for several ACSR in terms of parameters  $Y_b$  and  $f_{y_{max}}$ .

103 For  $f_{y_{max}}$  controlled fatigue tests, EPRI suggested endurance limits of 149 mm/s and 118 mm/s for single-layer and  
104 multilayer ACSR, respectively. Integrated in eq. (2.7), these limits result in a stress endurance limit  $\sigma_a$  of 22 MPa for both  
105 ACSR types. For fatigue tests performed with imposed  $Y_b$  values, the EPRI survey defined endurance limits of 0.5 to 1.0 mm  
106 for single-layer, and of 0.2 to 0.3 mm for multilayer, ACSR, respectively. Integrated in eq. (2.4), these evaluations lead to  
107 stress endurance limits  $\sigma_a$  of 22.5 MPa and 8.5 MPa for single-layer and multilayer, ACSR, respectively.

108 A comparison of the obtained  $\sigma_a$  estimates shows that the values are coherent for the single-layer ACSR, whereas for  
109 multilayer ACSR, an evident discrepancy appears between the  $Y_b$  and  $f_{y_{max}}$  predictions. This may be attributed to the *PS*  
110 model underlying assumptions which cannot account for wire strand kinematics.

111 In reality, the  $\sigma_a$  amplitudes derived from this idealized model should only be viewed as an indicator that is well  
112 correlated with experimental measurements of conductor fatigue life [16]. Expressing the conductor fatigue performance in  
113 terms of parameters  $Y_b$  and  $f_{y_{max}}$  is nevertheless common practice [4].

114 It is also worth mentioning that these evaluation approaches decouple the endurance limits from the stress causing  
115 fretting damages, and therefore, prevent a clear definition of the relationship between wind-induced vibrations and  
116 conductor fretting fatigue damages [15].

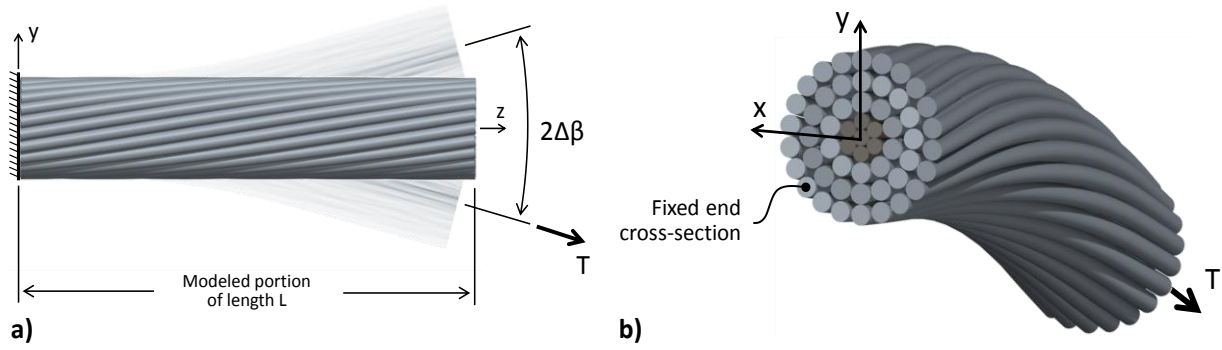
117 A refined conductor modeling approach should therefore be useful for obtaining  $\sigma_a$  estimations which better reflect the  
118 physics of the wire strand.

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120 **3. Finite element modeling approach**

121 In the following subsection, FE modeling strategy developed in Part I is applied to the ACSR alternating bending stress  
122 problem.

123  
124 *3.1. Model construction*

125 The model considers the conductor as rigidly clamped at one end. The other end undergoes fully reversed angular  
126 fluctuations of  $\Delta\theta$  amplitude under a constant axial tension  $T$  (Fig. 3a).



**Fig. 3 - FE model configuration (a) and Wire geometric configuration at fixed end (b)**

The model includes a strand section of length  $L$ . To facilitate the numerical-analytical  $\sigma_a$  comparison, the FE models are constructed such that at the fixed end, one of the wire cross-section centers of each strand layer is positioned on the  $y$ -axis (Fig. 3b).

### 3.2. Boundary conditions and load configuration

The nodes at both end sections are fully coupled with the associated node located at the center core wire. All DOF are constrained at the fixed end, whereas at the free end, only the  $x$  displacements and rotations about the  $z$ - and  $y$ -axes are blocked. The axial load ( $T$ ) is first applied in the horizontal direction. The Aeolian vibrations are then introduced through reorientations of  $T$  at  $\pm\Delta\beta$ . Angular variations of  $\Delta\beta$  are defined in terms of  $f_{y_{\max}}$  using eq. (2.5). They are gradually induced by increments of  $0.1^\circ$ . Finally, two deflection cycles are simulated in order to achieve a stabilized hysteresis loop (as defined in Part I).

### 3.3 Modeled ACSR

The following analysis considers four ACSR strands. The general and stranding properties of the studied conductor are given in Table 1 and Table 2, respectively [17]. In all simulations, the strand length  $L$  is fixed at 1000 mm.

**Table 1 - ACSR general properties**

Properties	ACSR 1/0	Drake	Crow	Bersfort
RTS (kN)	19.5	140.1	117.2	180.1
$m$ (kg/m)	0.216	1.628	1.369	2.370
$E_{\text{alum.}}$ (GPa)	69	69	69	69
$E_{\text{steel}}$ (GPa)	207	207	207	207
$EI_{\min}$ (Nm <sup>2</sup> )	3.9	43.4	18.1	61.6
$EI_{\max}$ (Nm <sup>2</sup> )	24.5	1495	1146	3827

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**Table 2 - ACSR stranding properties**

Layer	$n_i$	$d_i$ (mm)	E (GPa)	$\alpha_i$ (°)
ACSR 1/0				
Core	1	3.37	207	-
1	6	3.37	69	6
ACSR Drake				
Core	1	3.45	207	-
1	6	3.45	207	5.8
2	10	4.44	69	10.7
3	16	4.44	69	12.9
ACSR Crow				
Core	1	2.92	207	-
1	6	2.92	207	6.34
2	12	2.92	69	10.61
3	18	2.92	69	11.19
4	24	2.92	69	12.49
ACSR Bersfort				
Core	1	3.32	207	-
1	6	3.32	207	6.2
2	10	4.27	69	9.7
3	16	4.27	69	10.7
4	22	4.27	69	11.7

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### 3.4 Inter-wire contact modeling

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### 3.5. Numerical analysis of ACSR strand submitted to bending loads

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### 3.6. Validation of the modeling approach

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The following compares the simulation results to the experimental measurements published by Lévesque et al. [18]. In that reference paper, the authors tested three different ACSR types: Drake, Crow and Bersfort. The test bench is described in [19]). The conductors were excited at various controlled modes and  $y_{max}$  amplitudes under axial tension (T) levels

corresponding to: 15%, 25% and 35% of the conductor RTS (Rated Tensile Strength). The conductor peak-to-peak deflection amplitudes were measured at multiple locations: at 89 mm from the clamp edge ( $Y_b$  position), as well as at 45 mm ( $Y_{b45}$ ), 178 mm ( $Y_{b178}$ ), and 267 mm ( $Y_{b267}$ ).

The experimental configurations were reproduced in the model. Fig. 3a shows the rigid fixed-end conditions integrated in the model to reproduce the square-faced clamp of the experimental system. In addition, in order to obtain precise global trends, the simulations were not limited to the tested  $f_{y_{max}}$ , but rather, twenty  $\Delta\beta$  values, ranging from 0 to about  $2.0^\circ$ - $2.5^\circ$ , were evaluated for each tension level. Fig. 4 compares the numerical solutions to the Lévesque et al.'s [18] measurements. The comparison graphs include the peak-to-peak deflection amplitudes evaluated at 45 mm ( $Y_{b45}$ ), 89 mm ( $Y_{b89}$ ), and 178 mm ( $Y_{b178}$ ).

According to the experimental data shown in Fig. 4, the vibration mode-shape seems to have little influence on the conductor deflection close to the clamp. This observation is in line with the theory presented in Section 2.

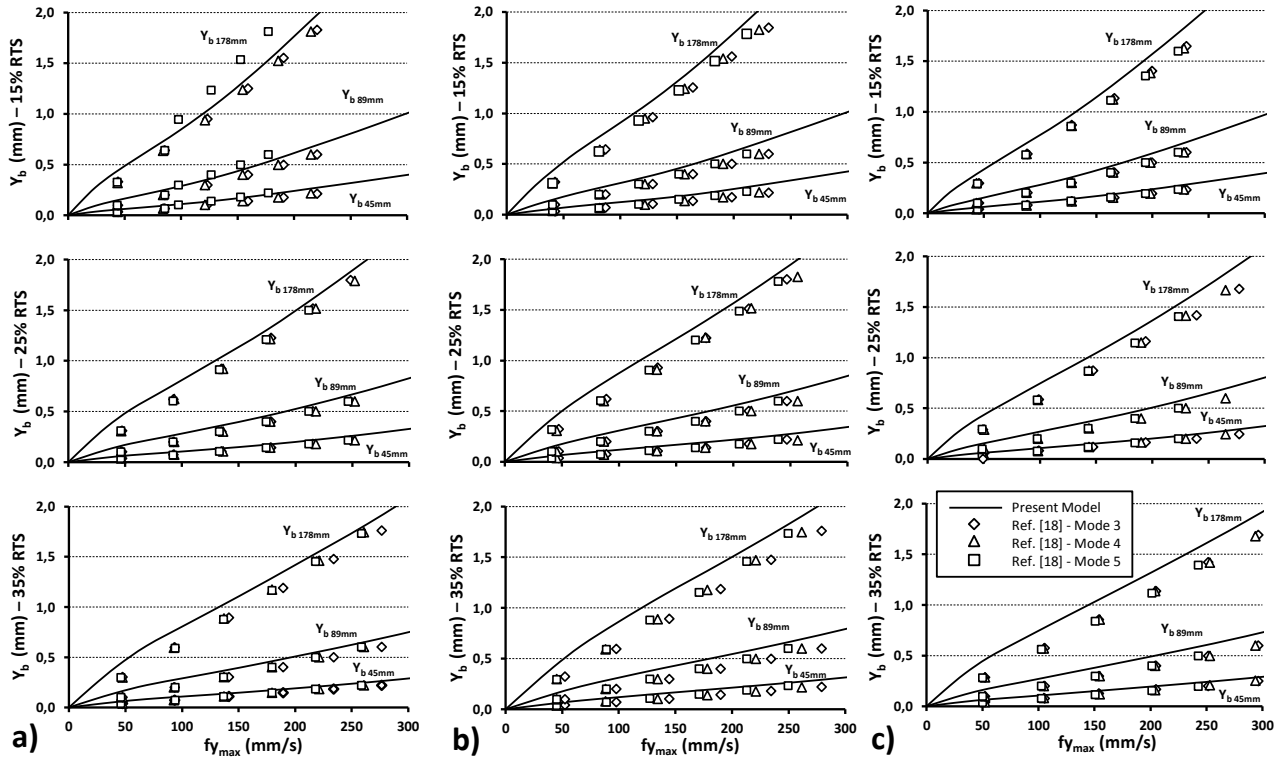


Fig. 4 -  $f_{y_{max}}$  vs.  $Y_b$  for ACSR Drake (a), Crow (b) and Bersfort (c) at  $T = 15\%$  RTS,  $25\%$  RTS and  $35\%$  RTS

The very high correspondence levels displayed in these figures confirm the validity of the proposed modeling approach. Globally, the FE model tends to slightly overestimate the conductor deflection. These differences may result from the assumed friction coefficients. Moreover, despite the apparent linear relationship between  $f_{y_{max}}$  and  $Y_b$  demonstrated by the



186 experimental measurements, the higher resolution obtained with the numerical simulations rather reveal a nonlinear  
 187 relation between these parameters.

#### 188 189 **4. Relation between $f_{y_{max}}$ and $Y_b$ criterion**

190 As previously mentioned, conductor fatigue performances are commonly defined in terms of  $f_{y_{max}}$  or  $Y_b$ , since both  
 191 represent measurable parameters. Therefore, depending on laboratory preferences or available equipment, fatigue curves  
 192 may sometimes be defined with  $f_{y_{max}}$  for a certain conductor model, and with  $Y_b$  for another. This often becomes  
 193 unmanageable. For example, for a given strand, it might be required to access the bending load severity for known  $f_{y_{max}}$   
 194 values, while the endurance limit is only defined for  $Y_b$ . Under such circumstances, EPRI [4] suggests experimentally  
 195 determining the  $f_{y_{max}}$  value that corresponds to the  $Y_b$  endurance limit. This test should also be performed at the  $Y_b$   
 196 endurance limit amplitude, since, as noted in Section 3, the ratio  $Y_b/f_{y_{max}}$  does not maintain a stable linear evolution.

197 The proposed modeling procedure offers an attractive alternative to experimental  $Y_b$ - $f_{y_{max}}$  evaluations. However, the FEA  
 198 model preparation and computational cost may quickly become prohibitive. It is therefore proposed to establish a  $Y_b$   
 199 predictive tool based on a factorial design approach and built on data generated with the present FE model.

200 The  $Y_b$  prediction model incorporates the influence of the key factors  $T$ ,  $f_{y_{max}}$  and  $\mu_a$ . The role of the adhesive coefficient  
 201 of friction  $\mu_a$  was indeed shown to be significant in Part I. Assuming quadratic variation effects of these parameters, the  
 202 model is built from a three-level ( $3^3$ ) factorial design. The selected factor  $T$  levels correspond to the usual fatigue data ranges  
 203 [4]. On the other hand, the  $\mu_a$  levels refer to values examined in Part I. For optimal Aeolian vibration coverage, the  $f_{y_{max}}$   
 204 range goes from 10 to 600 mm/s. Finally, to account for the  $Y_b$  curvature changes shown in Fig. 4, the  $f_{y_{max}}$  interpolation  
 205 space is further subdivided into two sub-domains ( $\Omega_1$ ,  $\Omega_2$ ). This subdivision improves the precision of the prediction model.

206 Table 3 presents the factors and the corresponding levels for each  $\Omega_i$ .

207  
208 **Table 3 - Factors and levels for the interpolation domains ( $\Omega$ )**

$\Omega$	Levels	Factors		
		T (% RTS)	$f_{y_{max}}$ (mm/s)	$\mu$
$\Omega_1$	1	15%	10	0.5
	2	25%	67.5	0.7
	3	35%	125	0.9
$\Omega_2$	1	15%	125	0.5
	2	25%	362.5	0.7
	3	35%	600	0.9

209 All factor combinations presented in Table 3 were simulated with the FE model described in Section 3. Considering each  $Y_b$   
 210 numerical solution as exact, the formulation of the prediction equation can be established based on Lagrange polynomial  
 211

shape functions, leading to eq. (4.1). Table 4 gives the eq. (4.1) coefficients  $c_i$  for  $\Omega_1$  and  $\Omega_2$  including three ACSR: Drake, Crow and Bersfort. Simulations conducted for the single-layer ACSR 1/0, demonstrated that the friction coefficient magnitude has no effect on the conductor deflection ( $Y_b$ ). Fig. 5a presents the model prediction for  $\mu_a = 0.5$  and  $\mu_a = 0.9$ . This plot shows that the estimates are perfectly matching. In contrast, Fig. 5b shows the presence of interactions between  $T$  and  $f_{y_{max}}$ .

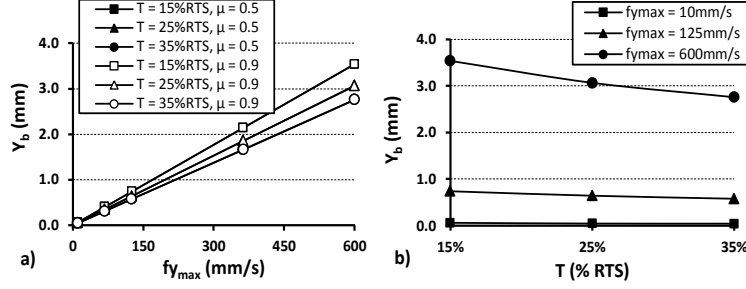


Fig. 5 – Single-layer ACSR 1/0  $Y_b$  variation with (a)  $f_{y_{max}}$  and (b)  $T$

Based on these observations, a two-factor factorial design ( $3^2$ ) considering a single  $f_{y_{max}}$  interpolation space between 10 and 600 mm/s appears to be better adapted. Table 5 presents the factors and the associated levels. The  $Y_b$  prediction equation reduces to eq. (4.2). Finally, Table 6 gives the  $c_i$  coefficients.

$$Y_b(T, f_{y_{max}}, \mu) = c_0 + c_1 T + c_2 f_{y_{max}} + c_3 \mu + c_4 T f_{y_{max}} + c_5 T \mu + c_6 f_{y_{max}} \mu + c_7 T f_{y_{max}} \mu + c_8 T^2 + c_9 f_{y_{max}}^2 + c_{10} \mu^2 + c_{11} T^2 f_{y_{max}} + c_{12} T^2 \mu + c_{13} T^2 f_{y_{max}} \mu + c_{14} T^2 f_{y_{max}}^2 + c_{15} T^2 f_{y_{max}}^2 \mu + c_{16} T^2 \mu^2 + c_{17} T^2 f_{y_{max}} \mu^2 + c_{18} T^2 f_{y_{max}}^2 \mu^2 + c_{19} T f_{y_{max}}^2 + c_{20} f_{y_{max}}^2 \mu + c_{21} f_{y_{max}}^2 \mu^2 + c_{22} T f_{y_{max}}^2 \mu + c_{23} T f_{y_{max}} \mu^2 + c_{24} T \mu^2 + c_{25} f_{y_{max}} \mu^2 + c_{26} T f_{y_{max}}^2 \mu^2 \quad (4.1)$$

Table 4 – Eq. (4.1)  $c_i$  coefficients

$c_i$	$\Omega_1$ ( $10 \leq f_{y_{max}} < 125$ mm/s)			$\Omega_2$ ( $125 \leq f_{y_{max}} < 600$ mm/s)		
	Drake	Crow	Bersfort	Drake	Crow	Bersfort
$c_0$	4.147746E-02	-4.565770E-01	-4.362340E-02	-1.254874E-01	-2.922646E-01	-3.795748E-01
$c_1$	-4.723617E-01	4.332205E+00	3.313860E-01	1.507831E-01	3.452069E+00	3.678961E+00
$c_2$	5.100441E-03	5.369405E-02	8.102636E-03	6.203661E-03	1.551895E-03	7.534341E-03
$c_3$	-1.401715E-01	1.397398E+00	1.040552E-01	-3.678143E-01	2.183204E-01	3.012140E-01
$c_4$	-4.835680E-03	-4.787176E-01	-3.876932E-02	-1.078746E-02	1.775008E-02	-3.854482E-02
$c_5$	1.585521E+00	-1.317469E+01	-5.227516E-01	5.223163E+00	-6.759697E+00	-6.254691E+00
$c_6$	1.105159E-04	-1.510798E-01	-9.417180E-03	2.184925E-03	1.126607E-02	-6.234226E-03
$c_7$	-1.956418E-02	1.442860E+00	5.793084E-02	-4.654720E-02	-9.797880E-02	6.481300E-02
$c_8$	1.263917E+00	-8.793719E+00	-4.156919E-01	9.869259E-01	-1.115650E+01	-9.073408E+00
$c_9$	-1.417234E-06	-3.970819E-04	-2.789478E-05	4.427541E-07	9.539370E-06	-1.847538E-06
$c_{10}$	1.211421E-01	-9.532355E-01	-1.305763E-02	4.109342E-01	-3.740889E-01	-4.913690E-01
$c_{11}$	-8.724118E-03	9.688959E-01	4.780175E-02	-1.467713E-06	-2.523364E-02	8.485980E-02
$c_{12}$	-4.209011E+00	2.664189E+01	1.982775E-01	-1.142012E+01	3.149248E+01	2.159976E+01
$c_{13}$	7.066455E-02	-2.920210E+00	-2.335847E-02	1.074735E-01	1.171207E-01	-1.958073E-01
$c_{14}$	7.909762E-05	-7.987134E-03	-3.584705E-04	2.704386E-05	1.171207E-04	-1.008411E-04
$c_{15}$	-3.341622E-04	2.427036E-02	2.784388E-04	-1.671228E-04	-3.387239E-04	2.883350E-04
$c_{16}$	3.550401E+00	-1.826174E+01	7.102872E-01	8.099565E+00	-2.795063E+01	-2.028933E+01
$c_{17}$	-6.745032E-02	2.000862E+00	-7.388414E-02	-7.830439E-02	-3.411320E-02	1.932788E-01
$c_{18}$	3.288162E-04	-1.672272E-02	5.100107E-04	1.245023E-04	1.771708E-04	-2.833172E-04
$c_{19}$	-1.385926E-05	3.844633E-03	2.524134E-04	-6.126250E-06	-7.078006E-05	3.637259E-05
$c_{20}$	-2.862127E-06	1.197747E-03	4.547789E-05	-4.888257E-06	-2.555857E-05	7.396101E-06
$c_{21}$	5.328412E-06	-8.338755E-04	2.317406E-06	5.605670E-06	1.616382E-05	-8.763715E-06
$c_{22}$	8.630541E-05	-1.170395E-02	-4.095632E-04	6.936048E-05	2.122026E-04	-9.777642E-05

$c_{23}$	2.153838E-02	-9.861089E-01	1.165334E-02	4.002149E-02	4.845359E-02	-7.321415E-02
$c_{24}$	-1.355614E+00	9.007796E+00	-1.220868E-01	-4.250385E+00	7.742336E+00	7.448899E+00
$c_{25}$	-1.187469E-03	1.038677E-01	1.113826E-03	-3.540463E-03	-7.020385E-03	6.325457E-03
$c_{26}$	-9.738842E-05	8.071674E-03	-8.651218E-05	-5.998786E-05	-1.238360E-04	1.078847E-04

**Table 5 - Factors and levels for the single-layer ACSR 1/0 case**

$\Omega$	Levels	Factors	
		T (% RTS)	$f_{y_{max}}$ (mm/s)
$\Omega_1$	1	15%	10
	2	25%	305
	3	35%	600

**Table 6 - Eq. (4.2)  $c_i$  coefficients for single layer ( $10 \leq f_{y_{max}} < 600$  mm/s)**

ACSR 1/0 $c_i$					
$c_0$	3.714370E-02	$c_3$	-1.029715E-02	$c_6$	7.059738E-03
$c_1$	-3.401287E-01	$c_4$	6.386957E-01	$c_7$	-4.013036E-06
$c_2$	7.276425E-03	$c_5$	4.069943E-07	$c_8$	9.033446E-06

$$Y_b(T, f_{y_{max}}) = c_0 + c_1 T + c_2 f_{y_{max}} + c_3 T f_{y_{max}} + c_4 T^2 + c_5 f_{y_{max}}^2 + c_6 T^2 f_{y_{max}} + c_7 T f_{y_{max}}^2 + c_8 T^2 f_{y_{max}}^2 \quad (4.2)$$

In order to validate the precision of the  $Y_b$  prediction equation (eqs. (4.1) and (4.2)), Table 7 and Table 8 present a validation plan testing all mid-level factor combinations.

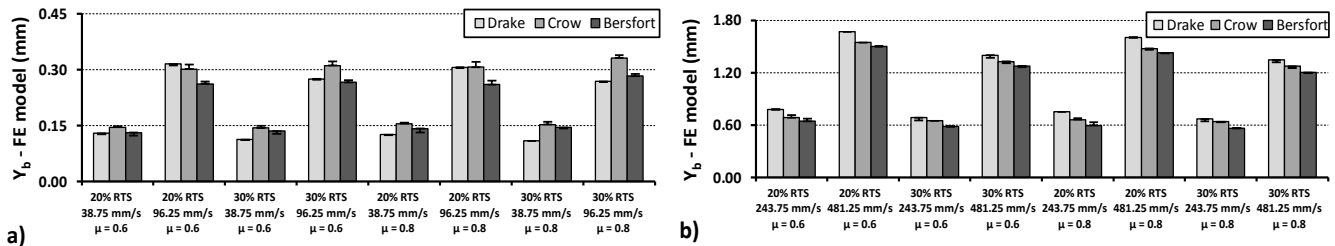
**Table 7 - Validation plan (single layer ACSR) factor values**

Mid levels	Factors	
	T (% RTS)	$f_{y_{max}}$ (mm/s)
1	20%	157.5
2	30%	452.5

**Table 8 - Validation plan (multilayer ACSR) factor values**

Mid levels	Factors		
	T (% RTS)	$f_{y_{max}}$ (mm/s)	$\mu$
1	20%	38.75	0.6
2	30%	96.25	0.8
3	-	243.75	-
4	-	481.25	-

These combinations were also simulated with the FE model defined in Section 3. Figures 6 and 7 compare the eqs. 4.1 and 4.2  $Y_b$  prediction values to the FE evaluations. The graphs also include the prediction error bars.



**Fig. 6 - Comparison of  $Y_b$  values obtained from eq. (4.1) (a)  $\Omega_1$  and (b)  $\Omega_2$  to FE evaluations with error bars**

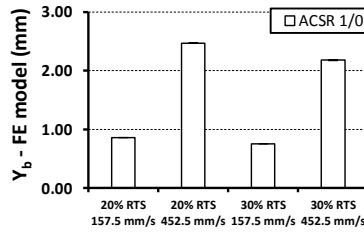


Fig. 7 - Comparison of  $Y_b$  values obtained from eq. (4.2) (single-layer ACSR 1/0) to FE evaluations with error bars

Compared to the FE solutions, the prediction equations (eqs. 4.1 and 4.2) demonstrate a high level of correspondence.

For example, at  $T = 20\%$  RTS and  $f_{y_{max}} = 452.5$  mm/s, eq. (4.2) (single-layer ACSR 1/0) leads to practically null deviations; the maximum error is 0.008 mm or 0.33%. On the other hand, compared to the  $Y_b$  FE solution, the maximum absolute deviation shown by eq. (4.1) (multilayer ACSR) is 0.034 mm (ACSR Bersfort at  $T = 20\%$  RTS,  $f_{y_{max}} = 243.75$  mm/s and  $\mu = 0.8$ ), while the maximum relative deviation is -7.61% (ACSR Bersfort at  $T = 20\%$  RTS,  $f_{y_{max}} = 38.75$  mm/s and  $\mu = 0.8$ ).

The available conductor endurance limits derived from  $Y_b$  or  $f_{y_{max}}$  were predominantly evaluated for short radius or bell mouth commercial metallic suspension clamps (Fig. 8). On the other hand, the prediction model does not integrate the fixture shape in the simulations. Thus, the precision of the estimation obtained from eqs. 4.1 and 4.2 for practical conditions remains to be evaluated. To that end, Fig. 9 compares the  $Y_b$  prediction resulting from eq. (4.1) to experimental  $f_{y_{max}}/Y_b$  measurements extracted from published ACSR fatigue data: Drake [19–22], Crow [23,24] and Bersfort [19,25]. Additionally, to render the friction coefficient effect on  $Y_b$  estimation more visible, Fig. 9 includes the prediction curves generated when  $\mu_a = 0.5, 0.7$  and  $0.9$  at the aluminum-aluminum and aluminum-steel contacts. The steel-steel contact coefficient of friction remains fixed at 0.3.

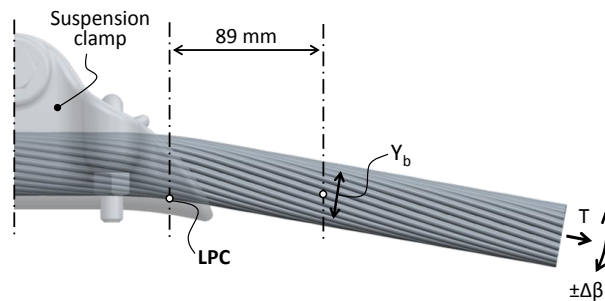
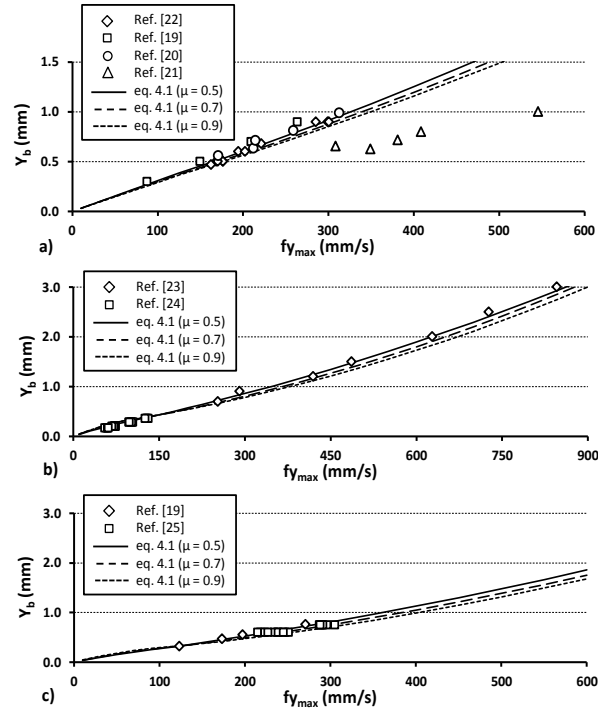


Fig. 8 - Commercial suspension clamp  $Y_b$  measurement



**Fig. 9 - Comparison of eq. (4.1) predictions with fatigue measurement of  $Y_b$  for ACSR at the exit of short radius metallic suspension clamps: (a) Drake, (b) Crow and (c) Bersfort tensioned at  $T = 25\%$  RTS**

The correlations shown in Fig. 9 are excellent for all three ACSR types. Even in Fig. 9b, where the prediction results from extrapolations outside the limits of the factorial design ( $f_{y_{max}} > 600$  mm/s), the estimations appear to be very close to the measurements. Fig. 9 also reveals that the  $\mu_a$  influence remains lower than the scattering of the experimental measurements. Therefore, it is considered that the  $\mu_a$  factor may be ignored and eliminated from the factorial design. This simplification results in a unique prediction formulation given by eq. (4.2) for all ACSR types. Moreover, since the  $Y_b$  estimations made when  $\mu_a$  is equal to 0.5 are slightly closer to the experimental measurements,  $\mu_a$  is fixed at 0.5 at the aluminum-aluminum and aluminum-steel contacts, while the steel-steel contact coefficient of friction remains unchanged at 0.3. Table 9 gives the final eq. (4.2)  $c_i$  coefficients for the Drake, Crow and Bersfort ACSR.

**Table 9 - Eq. (4.2)  $c_i$  coefficients for Drake, Crow and Bersfort ACSR when  $\mu_a = 0.5$**

$c_i$	$\Omega_1$ ( $10 \leq f_{y_{max}} < 125$ mm/s)			$\Omega_2$ ( $125 \leq f_{y_{max}} < 600$ mm/s)		
	Drake	Crow	Bersfort	Drake	Crow	Bersfort
$c_0$	1.677219E-03	3.813032E-03	5.862408E-03	-2.066610E-01	-2.766266E-01	-3.529718E-01
$c_1$	-1.850450E-02	-3.191341E-03	3.144200E-02	1.699768E+00	2.007805E+00	2.426234E+00
$c_2$	4.858832E-03	4.121085E-03	3.656021E-03	6.411008E-03	5.429836E-03	6.009825E-03
$c_3$	-9.233174E-03	-3.814655E-03	-6.707037E-03	-2.405569E-02	-1.912592E-02	-2.456169E-02
$c_4$	4.701172E-02	-3.820740E-02	-1.194490E-01	-2.698244E+00	-2.397915E+00	-3.376851E+00
$c_5$	-1.516194E-06	-6.677094E-06	-4.490862E-06	-5.999570E-07	8.010386E-07	-3.559112E-07
$c_6$	9.745577E-03	9.006427E-03	1.720596E-02	3.415919E-02	2.479839E-02	3.557543E-02
$c_7$	4.946343E-06	1.057589E-05	2.505030E-05	1.355703E-05	4.362259E-06	1.462084E-05
$c_8$	-5.779415E-06	-3.263413E-05	-8.943414E-05	-2.539196E-05	-7.948543E-06	-2.791612E-05

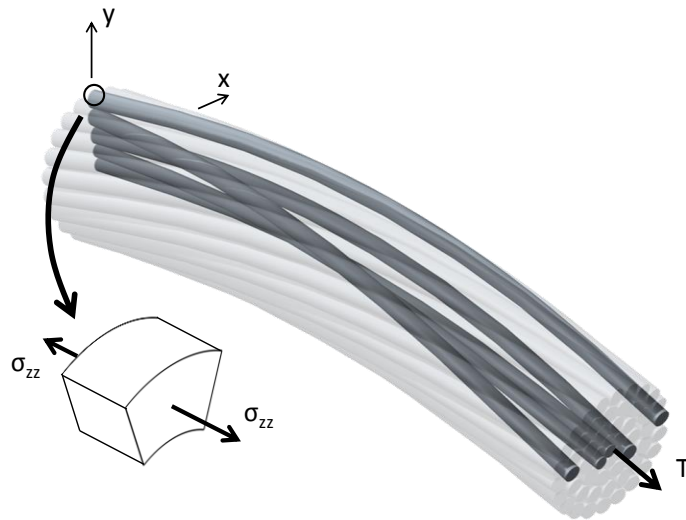
275 Finally, considering the close agreement shown in Fig. 9 between the prediction and the experimental measurements  
 276 conjointly with the original numerical model definition, which does not account for the clamping fixture shape, it may be  
 277 concluded that the short clamp radius has virtually no influence on the bending response evaluated at 89 mm from the Last  
 278 Point of Contact (LPC). The prediction tool formulated by eq. (4.2) and Tables 7 and 10 may thus be considered as offering  
 279 reliable evaluations of the bending amplitude of conductors supported by commercial suspension clamps. This simple model  
 280 provides an instant description of  $Y_b$  conditions produced by loads defined in terms of  $T$  and  $f_{y_{max}}$ .

## 281 5. Dynamic bending stress analysis ( $\sigma_a$ ) 282

283 The FE model developed in Part I can also provide immediate calculations of the alternating stress amplitude ( $\sigma_a$ ), and  
 284 thus, a direct assessment of the bending load severity via a fatigue damage analysis.

285 When only considering bulk stresses in configurations similar to that of Fig. 3 submitted to fatigue bending, the most  
 286 solicited region of each layer appears to be at the extreme fiber of the wires close to the clamped end, and aligned with the  
 287  $y$ -axis (Fig. 10).

288 The following makes use of the FE model described in Section 3 to determine the  $\sigma_a$  variations when the four conductors  
 289 of Table 1 are submitted to fluctuating  $Y_b$  amplitudes under  $T = 15\%$ ,  $25\%$  and  $35\%$  RTS. The  $\sigma_a$  evaluations describe the  $\sigma_{zz}$   
 290 stress differences (Fig. 10) produced by  $T$  between the two-limit angular positions  $-\Delta\beta$  and  $+\Delta\beta$ . The evaluation is carried out  
 291 for each aluminum layer.



292 Fig. 10 - Strand orientation for dynamic bending stress ( $\sigma_a$ ) evaluations  
 293  
 294

Fig. 11 to present the  $\sigma_a$  variations calculated for the  $Y_b$  ranging from 0 to 1 mm. The graphs also include the analytical evaluations produced with eq. (2.4), assuming  $El_{min}$  (eq. 2.3), and  $El_{max}$ .  $El_{max}$  is calculated with eq. (4.3) [26].

$$El_{max} = \sum_{i=1}^{nb \text{ layer}} n_i \left( E_i I_i \sin(\alpha_i) + \frac{A_i E_i R_i^2 \cos^3(\alpha_i)}{2} \right) + E_c I_c \quad (4.3)$$

In eq. (4.3),  $n_i$ ,  $A_i$ ,  $\alpha_i$  and  $R_i$  correspond to the number of wires, the cross-section area, the lay angle, and the layer  $i$  radius, respectively.  $E_c$  and  $I_c$  are the Young modulus and moment of inertia of the core wire.

A rapid inspection of the results shows that, on the one hand, the single-layer ACSR results in Fig. 11a demonstrate a perfect match between the FE model and the analytical solution, considering the  $El_{min}$  assumption, which is in agreement with the theory presented in Section 2. On the other hand, the multilayered configurations reveal responses completely different: for all three ACSR (Drake, Crow and Bersfort), at low  $Y_b$  amplitudes, the  $\sigma_a$  evaluations follow the  $El_{max}$  assumption, and progressively adopt a trend closer to  $El_{min}$  theory as  $Y_b$  increases. The stress response evolution systematically begins at lower  $Y_b$  amplitudes for the outer layer, and progresses toward the inner layers as  $Y_b$  intensifies. This process corresponds to a progressive interlayer partial deadhesion, which also generates a corresponding load transfer onto the inner layers (causing larger  $\sigma_a$ ). In other words, because of the interlayer sliding, the outer layer wires start bending about their own center fiber instead of respecting a group deformation about the conductor central axis.

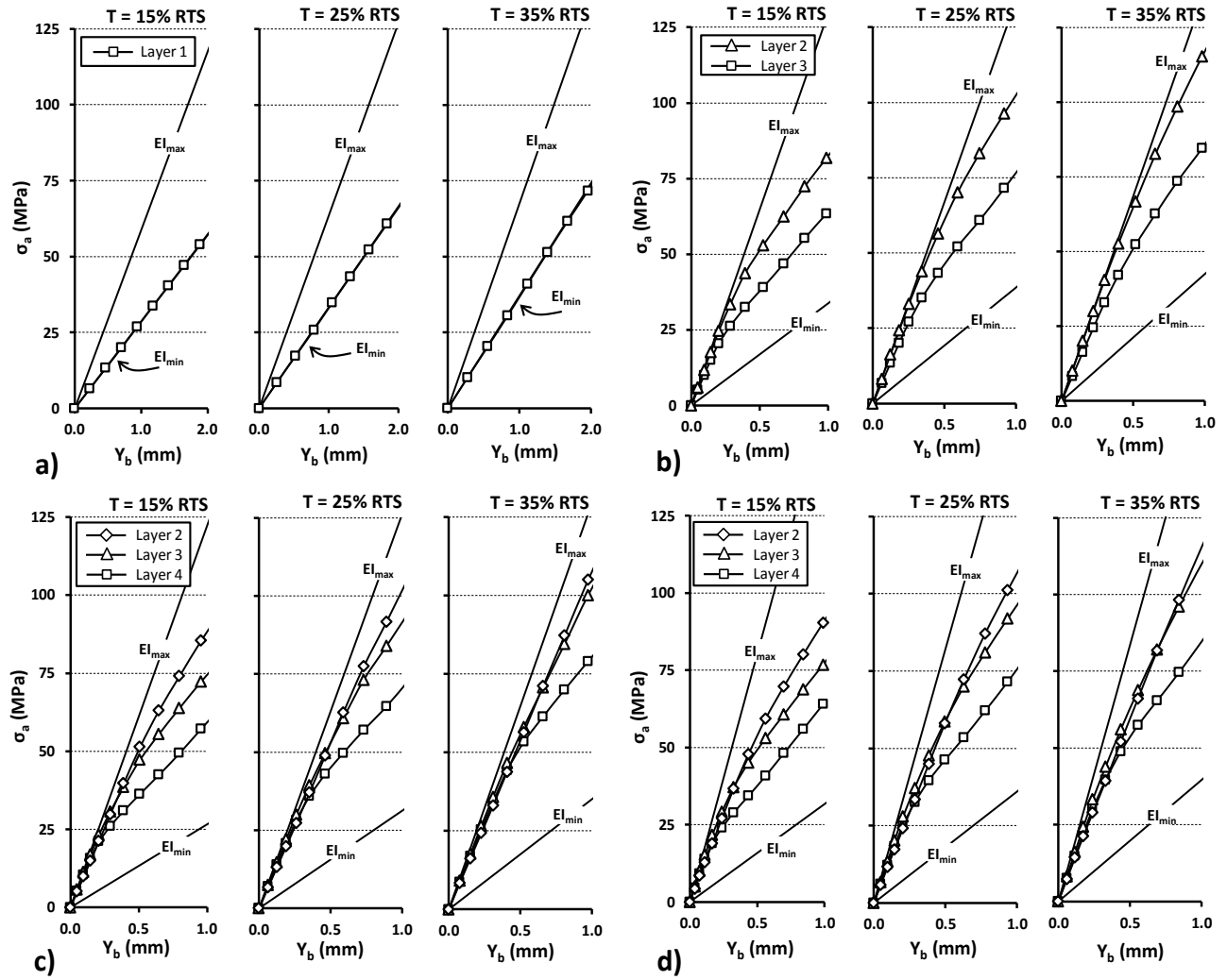


Fig. 11 - Dynamic bending stress - ACSR 1/0 (a), ACSR Drake (b), ACSR Crow (c) and ACSR Bersfort (d)

For the three analyzed multilayered ACSR, obvious signs of outer layer slip appear around  $Y_b$  amplitudes between 0.2 and 0.5 mm. Actually, a comparison of Fig. 11b) to d) shows that the exact  $Y_b$  onset level closely depends on the conductor tension ( $T$ ). Higher axial tensions  $T$  engender greater normal forces at the interlayer contact points, and consequently promote the internal conductor friction forces, thus favoring the wire adhesion.

### 5.1 Effects of coefficient of friction ( $\mu$ )

Although the results of Section 4.2 revealed a negligible effect of  $\mu_a$  on  $Y_b$ , in view of the last descriptions, the situation may be different for  $\sigma_a$ . To illustrate the influence of  $\mu_a$  on  $\sigma_a$ , Fig. 12 presents the variation of  $\sigma_a$  calculated with different  $\mu_a$  values for the Drake ACSR over a  $Y_b$  range of 0 to 2 mm.



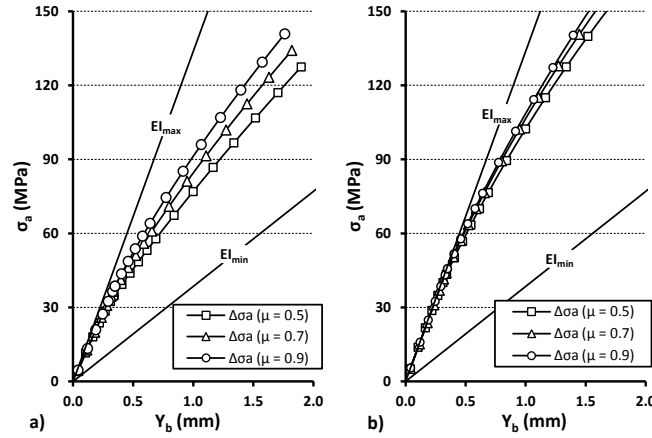


Fig. 12 - Variation of  $\sigma_a$  with  $Y_b$  in (a) outer layer 3 and (b) inner layer 2 for a Drake ACSR at  $T = 25\%$  RTS – influence of  $\mu_a$

As expected, since larger  $\mu_a$  values retard inter-wire slippage, and favor a grouped response of the conductor wires, Fig. 12a indicates that  $\sigma_a$  augments with  $\mu_a$  increases. This graph also shows increasing effects of  $\mu_a$  on  $\sigma_a$  with  $Y_b$  intensifications. Fig. 12b displays a similar response for the inner layer. However, because of the larger contributions of the outer layer wires conjointly provoked by  $\mu_a$  and  $Y_b$ , the difference between the prediction curves is of a lesser magnitude. Finally, Fig. 12 indicates that the  $\mu_a$  influence on the outer layer stress becomes significant for  $Y_b > 0.5$  mm. Thus, the friction coefficient effect may be less significant for smaller wind-induced conductor displacements associated with Aeolian vibrations. On the other hand, the coefficient of friction role becomes more significant for galloping transmission lines.

## 6. Fatigue life estimation ( $N_f$ )

Because of the complexity of the problem, current practice usually estimates the conductor fatigue life based on experimental data. In an attempt to establish a universal fatigue criterion for common conductors, the CIGRÉ study committee #22 [27] proposed the  $\sigma_a$  Safe Border Line eq. (6.1). This semi-empirical expression relates the outer layer alternate bending stress calculated with eq. (2.4) to the number of cycles to failure ( $N_f$ ). Actually, the safe line described by eq. (6.1) represents a conservative limit established from a collection of experimental measurements that were obtained through standardized fatigue tests involving conductors supported by metallic suspension clamps.

$$\sigma_a = \begin{cases} 450(N_f)^{-0.20} & \text{for } N_f \leq 1.56 \times 10^7 \\ 263(N_f)^{-0.17} & \text{for } N_f > 1.56 \times 10^7 \end{cases} \quad (6.1)$$

The proposed FE model also offers a valuable alternative to estimate  $N_f$ . The number of cycles to failure may be calculated from the wire bulk stresses of the outer layer extracted from the numerical simulations for given  $Y_b$  values, and

integrated into any plain fatigue criterion. Obviously, this approach does not explicitly account for the fretting damage contribution.

Adopting a basic stress-life approach, the well-known Basquin relation (eq. 6.2) may be selected as the fatigue criterion. Parameters  $\sigma_f$  and  $b$  represent the material properties. Table 10 gives these properties for wires made of 1350-H19 aluminum [28].

$$\sigma_a = \sigma_f \left( 2N_f \right)^b \quad (6.2)$$

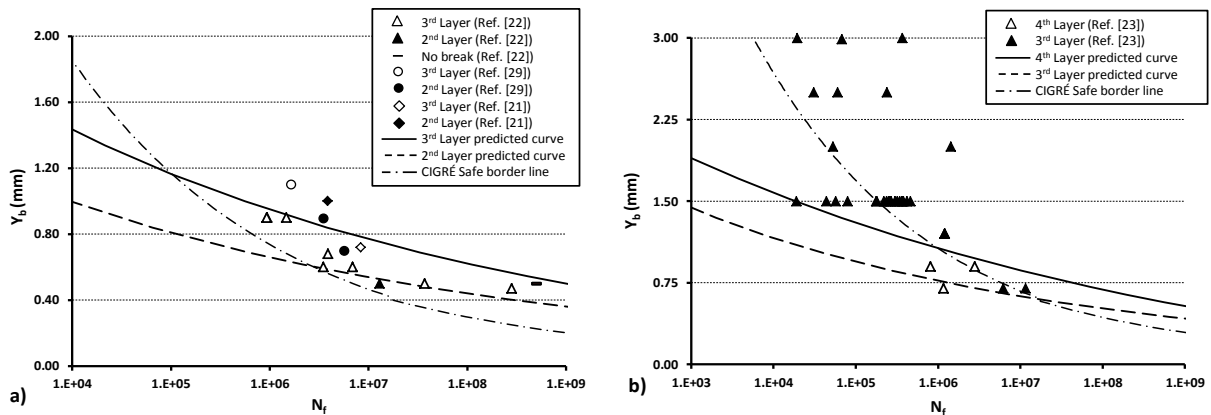
**Table 10 - 1350-H19 aluminum properties**

$\sigma_y$ (MPa)	$\sigma_u$ (MPa)	$\sigma_r$ (MPa)	$\epsilon_r$	$b$	$c$
167	187	204	0.274	-0.07	-0.5

The following fatigue life evaluations consider the wire stresses calculated at the critical location of the layers illustrated in Fig. 10. Therefore, they produce fatigue life estimates  $N_f$  controlled by the failure of the first wire of the layers.

The life predictions generated with the proposed approach are compared below with several published experimental fatigue results extracted from the literature for Drake and Crow ACSR. In all cases, the conductors were tensioned at 25% of their RTS, and supported by fixed short radius metallic suspension clamps.

Fig. 13 compares the numerical/experimental fatigue life evaluations obtained for both conductors. To preserve the data presentation form adopted in the references, the graphs display the results with  $Y_b$  as the damage criterion. They include the FE model-predicted fatigue curves for the two outermost aluminum layers, the experimental data, as well as the Safe Border Line (eq. (6.1)). The reported experimental  $N_f$  correspond to cycle numbers at the first wire break. Therefore, for the sake of clarity, the graphs also indicate the wire layer of the first experimental breakage.



**Fig. 13 -  $Y_b$ - $N_f$  1<sup>st</sup> break for Drake ACSR (a) and Crow ACSR (b) with prediction curves derived from the stress-life eq. (6.2)**

Fig. 13 evidences the scattering present in the experimental data. In fact, this is inherent to stranded conductor experimental investigations, where any slight dimensional, positioning or load variations may significantly affect the local contact conditions. All in all, the eq. (6.2) predicted lives do not wholly reflect the general trends described by the experimental data. Actually, the FE model predictions appear to be in better agreement with the experimental data at low  $Y_b$ . On the other hand, the CIGRÉ Safe Border Line offers relatively good conductor life estimations.

Fig. 11b) and c) show that at  $Y_b$  values above 0.75 mm,  $\sigma_a$  becomes very significant, and may presumably cause localized plastic deformations at the different stress risers. Therefore, to improve the model and account for plastic deformation effects, it is proposed to replace the Basquin relation (eq. 6.2) with the also well-known Coffin-Manson fatigue relation (eq. 6.3). In this equation, the variable  $\varepsilon_a$  is the measured or calculated strain amplitude at the critical location, and  $\varepsilon_f$  and  $c$  are material parameters already introduced in Table 10:

$$\varepsilon_a = \frac{(\sigma_f)}{E} (2N_f)^b + \varepsilon_f (2N_f)^c \quad (6.3)$$

The Fig. 13 fatigue prediction curves were recalculated with eq. (6.3). Fig. 14 compares the new estimates with the experimental measurements and the Safe Border Line results.

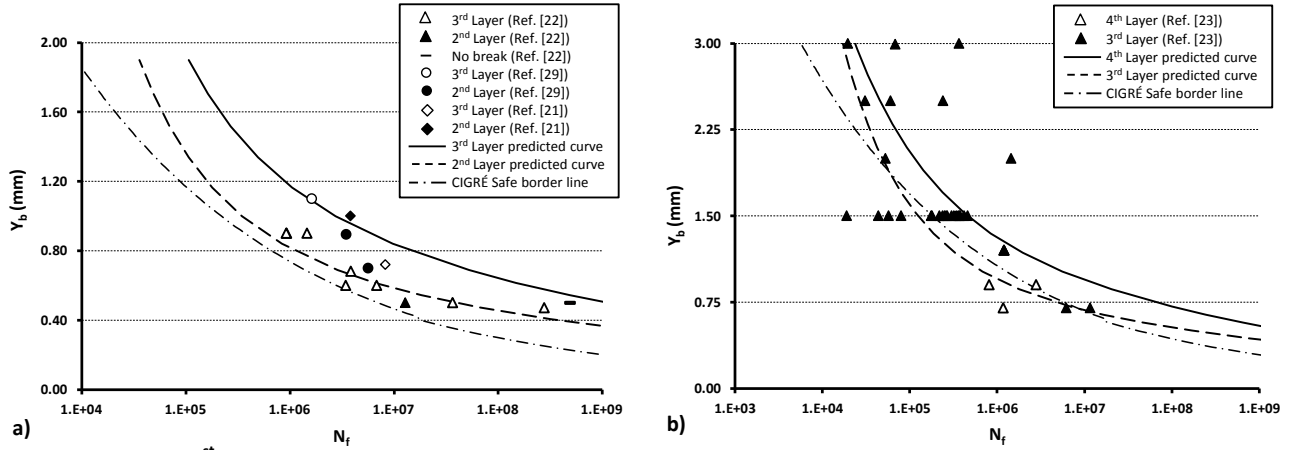


Fig. 14 -  $Y_b$ - $N_f$  1<sup>st</sup> break for (a) Drake ACSR and (b) Crow ACSR with prediction curves derived from the strain-life eq. (6.3)

Fig. 14 indicates that the addition of the plastic deformation effect improves the model prediction quality for both the Drake and Crow ACSR. Indeed, now the predicted life curves better correlate with the experimental measurements. In particular, the Crow ACSR graph of Fig. 14b shows that the predictions are now also excellent at very high  $Y_b$ . This is certainly an indication of the lesser role of the fretting mechanisms in fatigue damage induced under very high  $Y_b$ . In practice, high  $Y_b$  amplitudes are usually associated with galloping events.

385 The experimental data included also reveal that a majority of the first wire failure occurs at the outer layer for the Drake  
386 ACSR, while it is the opposite for the Crow ACSR, where the wires of the inner layer seem to be the most susceptible to  
387 fracture. This is presumably due to the lower  $Y_b$  of the fatigue test conditions maintained for the Drake ACSR, as compared to  
388 the Crow ACSR. Indeed, greater deflection amplitudes favor the slippage of the outermost wire, and thus reduce the load  
389 they support. The layers underneath consequently sustain higher stress levels, resulting in more rapid fatigue crack growth.  
390 On the other hand, at low  $Y_b$ , the wires of the outer layer share a larger part of the total load and are more severely affected  
391 by the fatigue crack propagation than the inner layers.

392 Fig. 14 also shows that in all cases, the FE model predicts a first wire break in the inner layer, which is not in perfect  
393 agreement with the experimental measurements. On the other hand, it is consistent with the numerical results of Fig. 11b)  
394 and c). This difference with the experimental measurements may be explained in part by underestimated friction coefficients  
395 ( $\mu_a$ ), especially in the clamped region. Indeed, Fig. 12 shows that higher  $\mu_a$  tend to increase the outer layer stress, while  
396 having less of an effect on the inner ones. The boundary conditions considered in the FE model in lieu of the real shape of the  
397 clamping fixture may also affect the outer layer slippage conditions.

398 In addition, the wear damages occurring at inter-wire contact points, which are not explicitly considered within the  
399 present fatigue formulation (eq.6), also certainly contribute to the differences between the experimental and numerical life  
400 predictions. The prevailing inter-wire contact regime may generate either sticking, gross slip or mixed conditions. While the  
401 sticking condition considerably reduces wear phenomena, gross slips lead to important wear rates. Therefore, since wear and  
402 fatigue are competing degradation modes, as supported by experimental measurements the mixed conditions associated  
403 with lower wear rates and rougher surfaces appears as the most detrimental regime [30]. Therefore, investigations focusing  
404 on the identification and the adaptation of a damage criterion better accounting for contact conditions, such as those  
405 published in Ref. [32,33], or considering wear laws as in Ref. [14], may contribute to refined the conductor life prediction  
406 obtained from the present FE model.

407 Finally, while the developed FE model precision may further be improved (with the addition of the suspension clamps and  
408 the incorporation of a refined damage criterion accounting for fatigue-wear interactions), the prediction accuracy remains  
409 quite high overall. The numerical results in Fig. 14 compare well with the experimental measurements, as well as with the  
410 semi-empirical estimations resulting from the CIGRÉ formula (eq. 6.1). Actually, because the omission of the clamp in the  
411 model reduces the calculation times considerably, the modeling strategy presented represents an effective trade-off.

## 412 7. Conclusion

413 The present paper made use of the FE modeling strategy developed in Part I to study the internal strain-stress conditions  
414 of ACSR conductors submitted to wind-induced loads. The first portion of the investigation examined the descriptors  $Y_b$  and  
415  $f_{y_{max}}$  used by researchers and industrial designers to evaluate load severity. The tests directly underlined the discrepancies  
416 between these descriptors, along with the difficulty in coupling them in conductor life prediction. The main objective of the  
417 second portion of the analysis was to evaluate an alternative way of investigating the conductor fatigue problem based on  
418 realistic stress/strain descriptions.

419 The study included four ACSR covering single- to four-layer configurations. Initial comparisons with reference  
420 experimental test data demonstrated the ability of the FE model to predict the bending deflection amplitudes  $Y_b$  in the near  
421 field of the clamped zone from the far-field vibration parameter  $f_{y_{max}}$ .

422 In order to circumvent the incompatibility existing between  $Y_b$  and  $f_{y_{max}}$ , and to provide a practical strategy for accurately  
423 relating them, a numerical evaluation of their responses integrated into a factorial design approach led to a multivariate  
424 prediction equation for  $Y_b$ . The resulting model provides instantaneous predictions of  $Y_b$  as a function of selected  $f_{y_{max}}$ ,  $T$  and  
425  $\mu_a$  values within an error level less than 8% when compared to a full 3D FE analysis. Additional comparisons of the predicted  
426  $Y_b$  with reference experimental measurements also demonstrated the applicability of the equation to conductors supported  
427 by commercial metallic suspension clamps. This simple model offers an easy way of converting and relating reference data  
428 established by various laboratories following specific standards and different research goals.

429 Because the practical descriptions of  $Y_b$  and  $f_{y_{max}}$  cannot depict the relationship connecting the experimental endurance  
430 limit and the strain-stress conditions causing fatigue damages, the study examined the merits of life predictions made from  
431 basic stress/strain damage analysis. Disregarding the effect of contact stresses, the investigation only considered the bulk  
432 stress/strain amplitudes of the wire to estimate the conductor residual life. Comparisons with experimental data of two  
433 multilayered ACSR indicated that the fatigue prediction curves established from the Coffin-Manson fatigue relation provides  
434 realistic evaluations of the service life of conductors submitted to both low amplitude deflections generated by wind-induced  
435 vibrations and high amplitude deflections resulting from line galloping.

436

437

438

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442

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